

ADDENDUM #3

May 4, 2012

- Re: Harrisburg Area Community College Gettysburg Campus Parking Lot Expansion Solicitation #RFB12-14
- From: Eastern PCM, LLC Construction Manager – HACC 645 N. 12th Street, Suite 200 Lemoyne, PA 17043

To: All Planholders

This Addendum is hereby made part of the Plans and Project Manual dated April 12, 2012 for the above referenced project. The provisions of this Addendum are intended to supplement the provisions of the Plans and Project Manual and/or supersede them where contradictory thereto.

This Addendum contains changes to the requirements of the Plans and Project Manual. Such changes shall be incorporated into the Plans and Project Manual and shall apply to work with the same meaning and force as if they had been included in the original Plans and Project Manual. Where this Addendum modifies a portion of a paragraph or phrase of the Project Manual, the remaining unmodified portion of the paragraph or phrase shall remain in force.

The conditions and terms of the Plans and Project Manual shall govern work described in this Addendum. Whenever the conditions of work, or the quality or quantity of materials or workmanship are not fully described in this Addendum, the conditions of work etc. included in the Plans and Project Manual for similar items of work shall apply to the work described in this Addendum. If no similar items of work are included in the Plans and Project Manual, the quality of material and workmanship shall be subject to the written acceptance of the Architect.

3.1 CHANGES TO THE PROJECT MANUAL

A. <u>Specification Section 16540 – SITE LIGHTING</u>

- a. DELETE paragraph 3.2.F in its entirety.
- b. Paragraph 1.2.E: DELETE "meter assembly and panelboard."
- c. Paragraph 1.2.G: DELETE "and installation."

B. Specification Section 02740 - Asphalt Paving

Paragraph 1.4.B: REVISE to read:

"The liquid asphalt Price Index value for this project (hereafter the "IB") shall be \$643.00 /ton as obtained from the Pennsylvania Asphalt Pavement Association (see www.pahotmix.org) for the month of May 2012 in Zone 1."

3.2 CHANGES TO THE DRAWINGS

- A. Drawing Sheet 7 CLARIFICATION: weed barrier shall be installed within all curbed islands and the areas shall be mulched.
- B. Drawing Sheet 8 CLARIFICATION: wood stakes, as shown on the landscape details, are acceptable.

3.3 CLARIFICATIONS

- A. The relocated fire hydrant shall be served by a 6" lateral connection.
- B. Substitutions requests can be submitted for review and approval after award of contract.
- C. Information regarding the existing electrical panel can be obtained by coordinating a site visit with the Construction Manager.

3.4 ATTACHMENTS

- A. Drawing 4 Grading and Utilities Plan
- B. Drawing 7 Lighting and Landscape Plan
- C. Drawing 8 Construction Details
- D. Pavement Design Analysis and Soil Classification Report

END OF ADDENDUM



Please sign and return this page, via fax, to Eastern PCM, LLC at (717) 233-1666 indicating receipt of this Addendum.

Harrisburg Area Community College Gettysburg Campus Parking Lot Expansion Solicitation #RFB12-14

Addendum #_____ has been received.

Company:

Print Company Name

Received By:

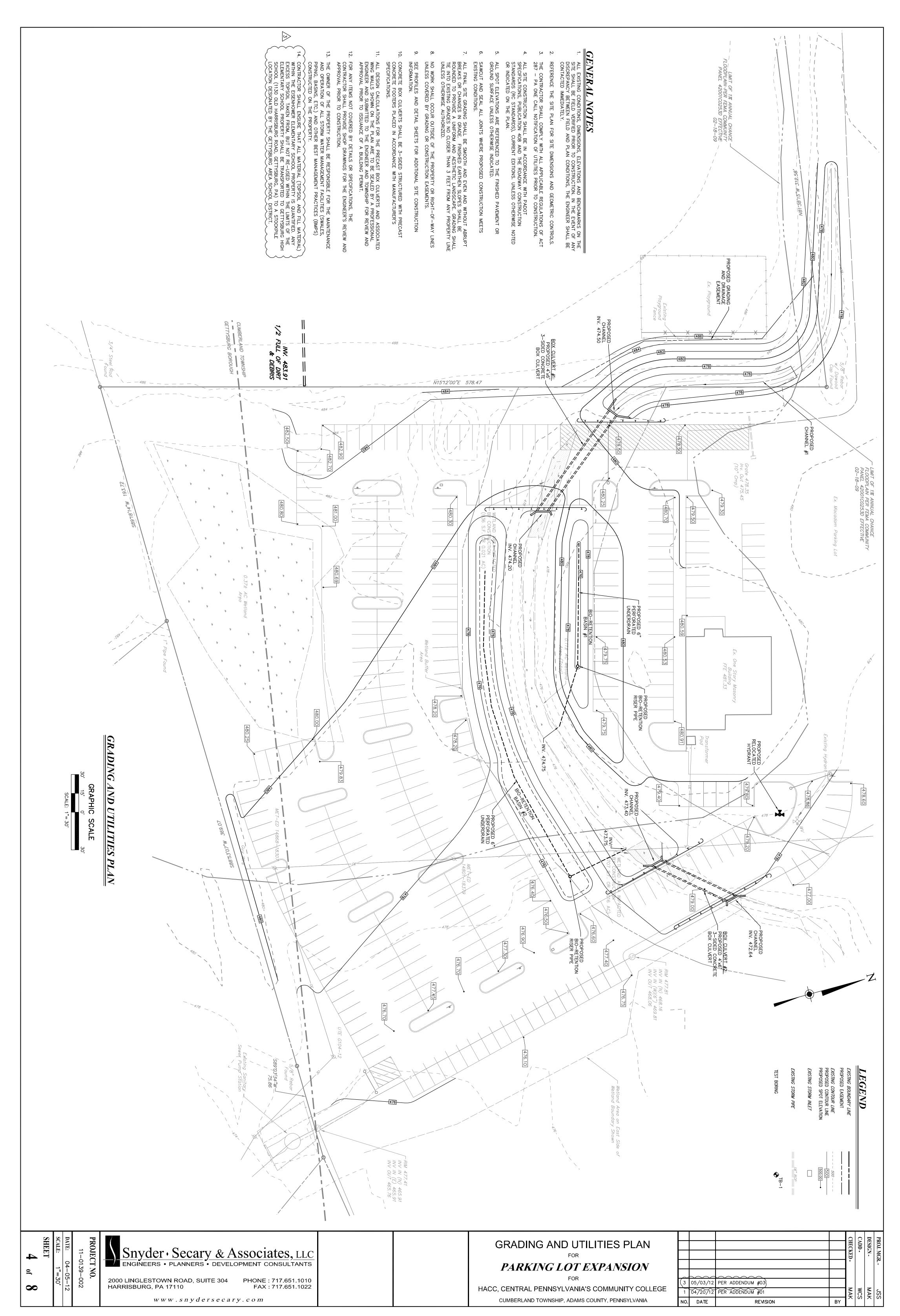
Print Name

Signature

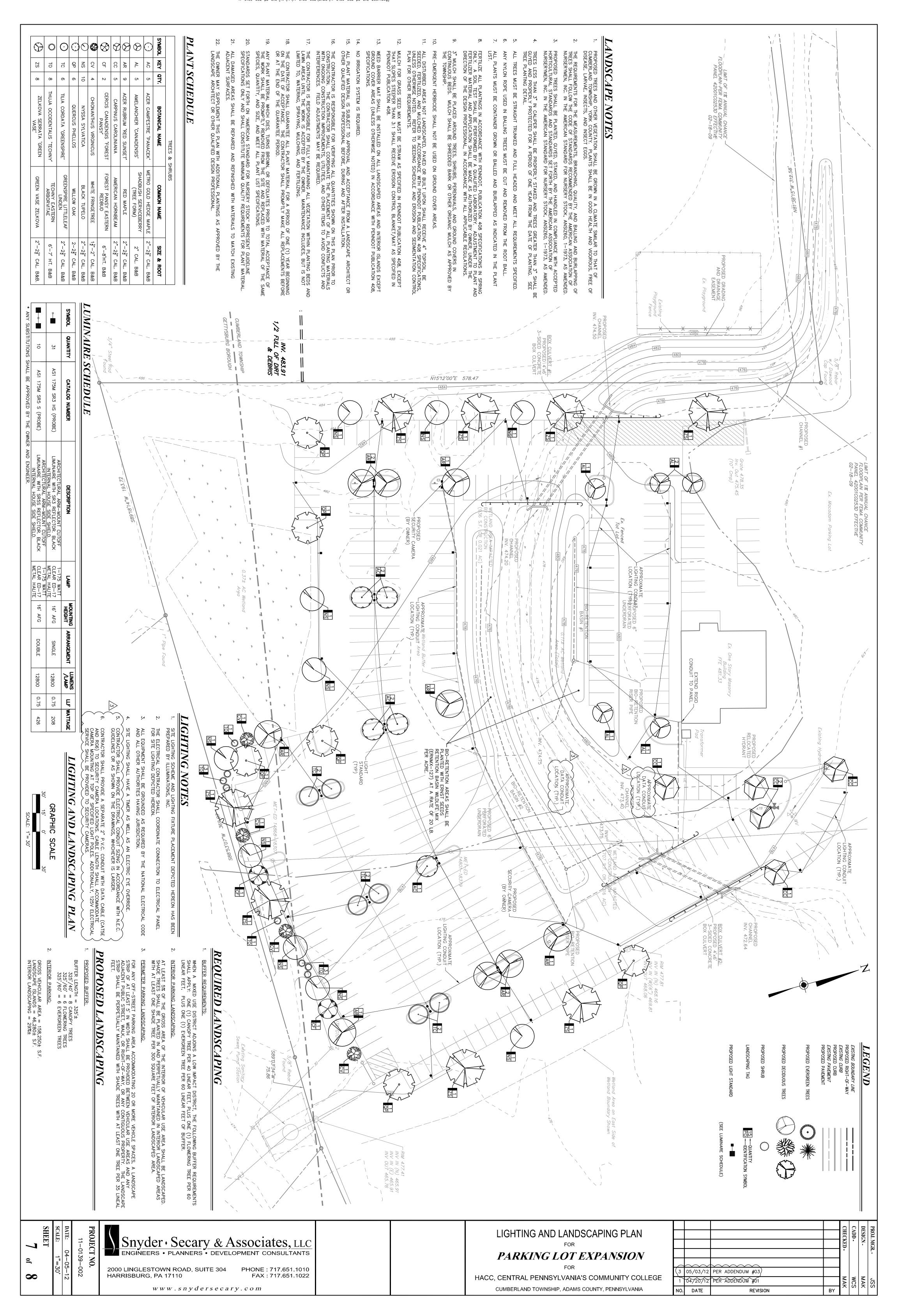
Date:

Please check one:

- We are bidding as a prime contractor
 - We are not bidding
 - We are a sub-contractor

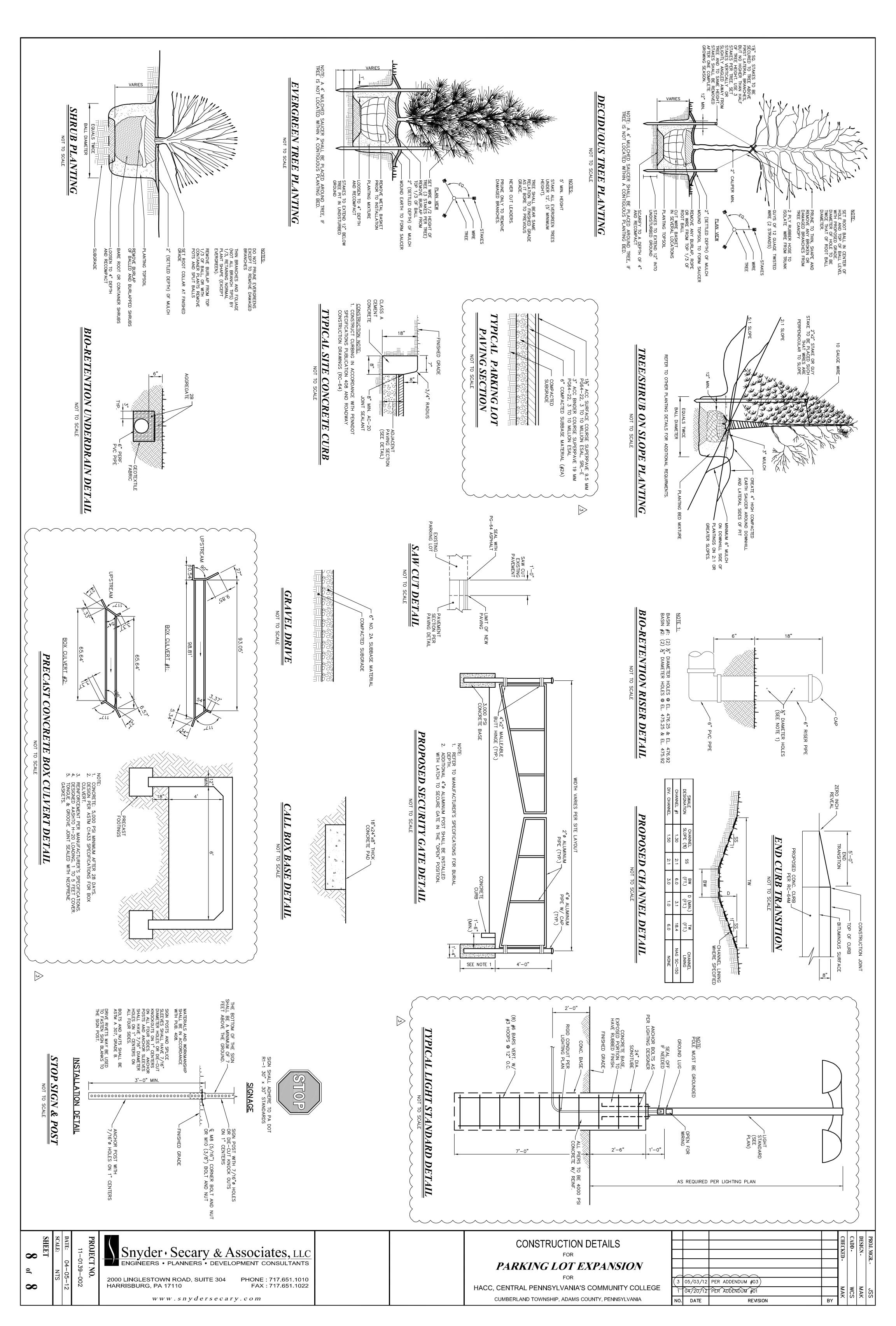


XREFS: 11-0139-002_xbase [H: \11\11-0139-002\CADD\11-0139-002_xbase.dwg] 11-0139-002-TB [H: \11\11-0139-002\CADD\11-0139-002-TB-CON.dwg] Legend [H: \11\11-0139-002\CADD\11-0139-002-pb-Legend.dwg] 11-0139-002_pbase [H: \11\11-0139-002\CADD\11-0139-002_pbase.dwg] 11-0139-002-pb-GnU [H: \11\11-0139-002\CADD\11-0139-002_pb-GnU.dwg]



DRAWING REFERENCE: H: \11\11-0139-002\CADD\LD\CONSTRUCTION DOCS\11-0139-002-07-LnL-2007-CON.dwg

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May 1, 2012



Mr. Mitchell A. Kemp, P.E. Snyder-Secary & Associates, LLC 200 Linglestown Road, Suite 304 Harrisburg, PA 17110

RE: Pavement Design Analysis Gettysburg HACC Parking Expansion Cumberland Township, Adams County, Pennsylvania Advantage Project No.: 120008501

Dear Mr. Kemp:

In accordance with your request, Advantage Engineers, LLC (Advantage) has completed a pavement design analysis for the proposed flexible pavement associated with the parking lot expansion for Gettysburg HACC in Cumberland Township, Adams County, Pennsylvania. This correspondence serves to transmit the results of our evaluation and our recommendations.

Two (2) bulk samples of on-site soil were retrieved from hand-excavated test pits, which extended to maximum depths of approximately 24 inches below existing site grades and subjected to the following testing:

•	Standard Classification Testing	-	ASTM D2487
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- Standard Proctor Analysis ASTM D698
- California Bearing Ratio (CBR) ASTM D1883

The results of these analyses are presented below in Table I. Graphical depictions of the gradation analyses, moisture-density curves from the standard proctor testing and the stress vs. strain curves from the CBR analyses are attached for review.

TABLE I

STANDARD PROCTOR & CALIFORNIA BEARING RATIO (CBR) ANALYSIS RESULTS											
SAMPLE NUMBER	SOIL CLASSIFICATION	MAXIMUM DRY DENSITY @ OPTIMUM MOISTURE CONTENT	CBR RESULTS (ASTM D1883)								
120008501-S1	Lean Clay with Sand (CL)	110.6 pcf @ 15.3%	3								
120008501-S2	Lean Clay with Sand (CL)	101.4 pcf @ 21.1%	5								

The flexible pavement section provided herein was designed in accordance with AASHTO Design Guide and is based on an average laboratory-determined CBR value of 4 for the subgrade soils. The design section has been determined for a design life of 20 years, with a reliability level of 85%, an overall standard deviation of 0.35, and a Terminal Service Index of 2.0.

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Mr. Mitchell A. Kemp, P.E. May 1, 2012 Page 2 of 3

Specific traffic loading information was not available at the time of this writing; therefore, it was estimated that the new parking lot expansion will receive 50,000 ESALs over its design life. Incorporating the above design life ESALs into the AASHTO flexible pavement design methodology yields a structural number (SN) of 2.3 for the proposed standard duty pavement section.

The thickness of the pavement section was determined using the following structural formula:

• $SN = a_1D_1 + a_2D_2 + a_3D_3$

where:

- a₁, a₂, a₃ = Structural coefficients for stabilized base, wearing course, and sub-base materials, respectively
- D₁, D₂, D₃ = Thickness of stabilized base, wearing coarse, and sub-base layers, respectively

The layer coefficients (a₁, a₂, a₃) used for the pavement design equation represent the ability of each material in the pavement section to support the design traffic loads. The elastic (resilient) modulus for each material in the pavement section (i.e. sub-base, base coarse, wearing coarse) is often used to establish the layer coefficient for these materials. Absent resilient modulus testing for each of the pavement components, the following values were assigned for this analysis.

- a₁=0.34
- a₂ = 0.44
- a₃ = 0.11

Based on the above structural number, the flexible pavement section was calculated to be the following:

9.5 mm Wearing Course	1.5 inches
19 mm Binder Course	3.0 inches
PennDOT 2A Aggregate Base Course	6.0 inches

Proper drainage will be an important consideration for the overall performance of the pavement design recommended above. We have assumed that proper grading to provide suitable runoff from the pavement surface and beyond the limits of the paved areas will be provided. **Pavements in cut areas may also require some under drainage considerations, which should be evaluated during construction.**

As minor cracking in the pavement section occurs with age, and if water is allowed to pond on the surface, seepage into the base-course material may weaken the subgrade, which can enhance degradation of the pavement section. Maintenance of this pavement will be critical to limiting its strength loss over the life of the pavement.

We recommend that the sub-base be placed as soon as possible after the subgrade has been approved. The asphalt should also be placed as soon as possible after the sub-base has been tested and approved. These recommendations are provided in an effort to help prevent the subgrade and the sub-base from being disturbed by weather and construction traffic. It will also help reduce the potential for the sub-base from becoming contaminated with soil. We also recommend that the flexible pavement section be prepared and placed according to PennDOT specifications. It should be noted that the pavement design has considered the standard loading for its intended use. The design does not consider construction traffic loadings which

Mr. Mitchell A. Kemp, P.E. May 1, 2012 Page 3 of 3

would make the section substantially more expensive. The general contractor and paving contractor should be advised that they must control the construction traffic so as to limit disturbance of previously approved subgrade, stone sub-base, and/or completed asphalt.

We trust that this is the information you require. Should you have any questions regarding the above information or if we may be of further assistance, please do not hesitate to contact our office.

Sincerely, Advantage Engineers, LLC

David J. Buckwalter Project Manager

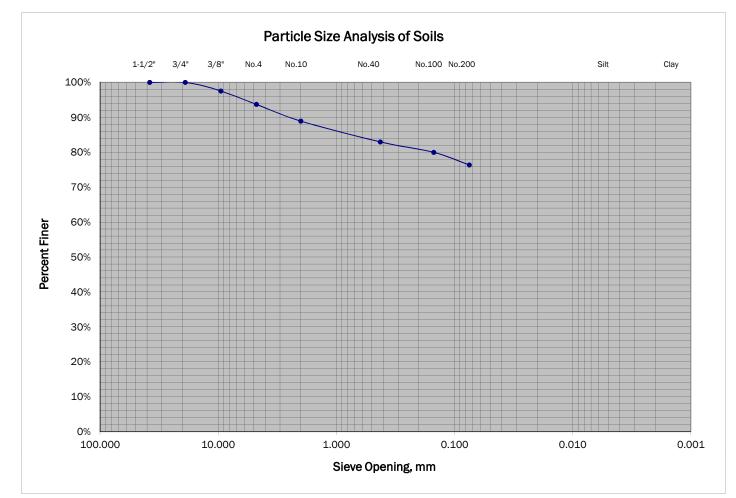
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Daniel R. Schauble, Jr Director of Geotechnical Services

Attachments: Standard Classification Reports Standard Laboratory Compaction California Bearing Ratio

Soil Classification Report

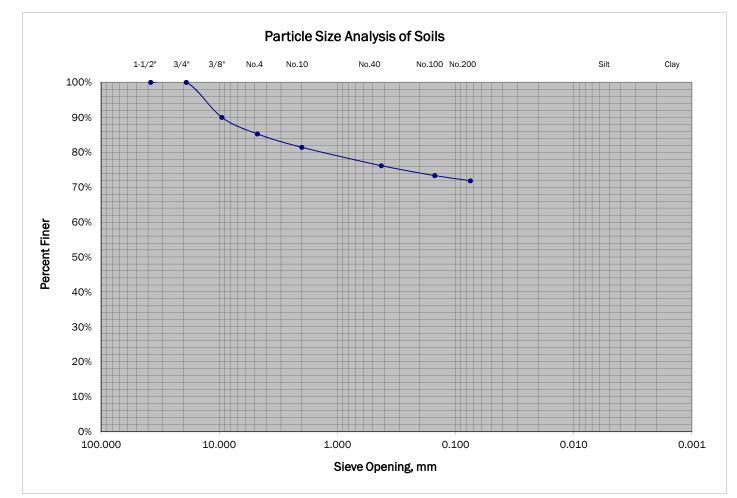
Per ASTM Designations D 2487 - 00 and D 2488 - 00



As-Received Moisture: 13.8%					Particle Size Distribution							
USCS Cla	ssification:	Lean Clay	with Sa	and - (CL)				US Standard Sieve Size Opening (mn) %Finer	
Gravel:	6.3%	Coarse:	0.0%			Fine:	6.3%	Coarse	1-1/2"	38.0	100.0%	
Sand:	17.3%	Coarse:	4.8%	Medium:	6.0%	Fine:	6.6%	GRAVEL	3/4"	19.0	100.0%	
Silt:		Clay:		C	olloids:			Fine	3/8"	9.50	97.5%	
Gravel De	escription:	Grey to re	ed subai	ngular to ang	gular				No. 4	4.75	93.7%	
								Coarse	No. 10	2.00	88.9%	
Sand Des	scription:	Red to gr	ey suba	ngular				Medium	No. 40	0.425	82.9%	
								SAND	No. 100	0.150	80.0%	
Consister	ncy: N/A			Dry Strength	n:	Medium		Fine	No. 200	0.075	76.4%	
Dilatancy	: Slow			Toughness:		Medium		Hydrometer	Silt Size	0.005		
Structure	: N/A			Cementatio	า:	N/A		Analysis	Clay Size	0.001		
								D ₆₀ :	D ₃₀ :	D ₁₀ :	Cu: Cc:	
Boring:								Atterberg Limits	LL: 31	PL: 23	PI: 8	
Sample:	S1		Depth:					Description:	Red brown			
Project:	Gettysburg	HACC										
								Remarks:	Sample #1			
Client:	Snyder, Seo	cary & Asso	ociates,	LLC.								
Advantag	e Project Nu	mber:		120008501				Report Date:	April 25, 2012			

Soil Classification Report

Per ASTM Designations D 2487 - 00 and D 2488 - 00



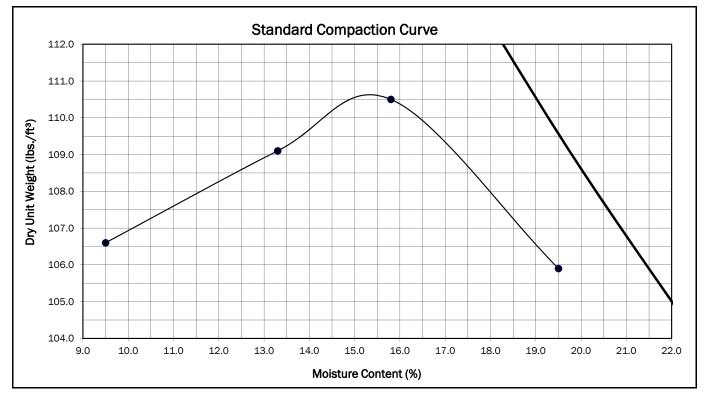
As-Receiv	ed Moisture	: 22.0%							Particle S	ize Distribution		
USCS Cla	ssification:	Lean Clay	with Sa	and - (CL)				US Standard	Sieve Size	Opening (mm)	%Finer	
Gravel:	14.8%	Coarse:	0.0%			Fine:	14.8%	Coarse	1-1/2"	38.0	10	0.0%
Sand:	13.4%	Coarse:	3.8%	Medium:	5.2%	Fine:	4.3%	GRAVEL	3/4"	19.0	10	0.0%
Silt:		Clay:		C	olloids:			Fine	3/8"	9.50	9	0.0%
Gravel De	escription:	Angular te	o suban	gular					No. 4	4.75	8	5.2%
								Coarse	No. 10	2.00	8	1.4%
Sand Des	cription:	Angular to	o suban	gular				Medium	No. 40	0.425	7	6.1%
								SAND	No. 100	0.150	7	3.3%
Consisten	ncy: N/A			Dry Strength	:	Medium	า	Fine	No. 200	0.075	7	1.8%
Dilatancy	: Slow			Toughness:		Medium	า	Hydrometer	Silt Size	0.005		
Structure	: N/A			Cementation	າ:	N/A		Analysis	Clay Size	0.001		
								D ₆₀ :	D ₃₀ :	D ₁₀ :	Cu:	Cc:
Boring:								Atterberg Limits	LL: 42	PL: 24	P	l: 18
Sample:	S2		Depth:					Description:	Red brown			
Project:	Gettysburg	HACC						1				
								Remarks:	Sample #2			
Client:	Snyder, Se	cary & Asso	ociates,	LLC.								
Advantag	e Project Nu	mber:		120008501				Report Date:	April 25, 2012			



Laboratory Compaction Characteristics Using Standard Effort Per ASTM Designation D 698 - 07, Method C ~ AASHTO Designation T 99 - 01, Method D

Date:	April 25, 2012		Project :	Gettysburg HACC		
Client:	Snyder, Secary & Asso	ociates. LLC	Advantage Project Number:	120008501		
Sample Description:	mple Description: Red brown		Sample ID:	120008501-S1		
Rammer Used:	Manual	Preparation Method:	Moist	As Received Moisture:	13.8%	

Test Data										
Point #1	Point #2	Point #3	Point #4							
116.7	123.6	127.9	126.6							
9.5	13.3	15.8	19.5							
106.6	109.1	110.5	105.9							
	Point #1 116.7 9.5	Point #1 Point #2 116.7 123.6 9.5 13.3	Point #1 Point #2 Point #3 116.7 123.6 127.9 9.5 13.3 15.8							



 Maximum Dry Unit Weight:
 110.6 lbs./ft.³
 Optimum Moisture Content:
 15.3 %

 The results stated on this report relate only to the material specifically identified.
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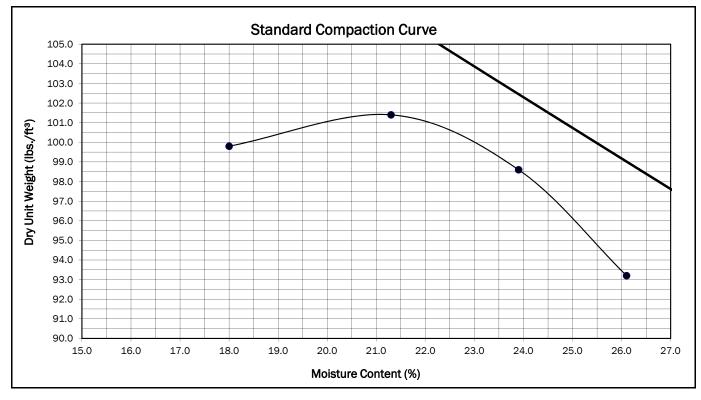
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Laboratory Compaction Characteristics Using Standard Effort Per ASTM Designation D 698 - 07, Method C ~ AASHTO Designation T 99 - 01, Method D

Date:	April 25, 2012		Project :	Gettysburg HACC			
Client:	Snyder, Secary & Asso	ociates. LLC	Advantage Project Number:	120008501			
Sample Description:	mple Description: Red brown		Sample ID:	120008501-S2			
Rammer Used:	Manual	Preparation Method:	Moist	As Received Moisture:	22.0%		

Test Data										
Point #1	Point #2	Point #3	Point #4							
117.8	123.0	122.1	117.6							
18.0	21.3	23.9	26.1							
99.8	101.4	98.6	93.2							
	Point #1 117.8 18.0	Point #1 Point #2 117.8 123.0 18.0 21.3	Point #1 Point #2 Point #3 117.8 123.0 122.1 18.0 21.3 23.9							



 Maximum Dry Unit Weight:
 101.4 lbs./ft.³
 Optimum Moisture Content:
 21.1 %

 The results stated on this report relate only to the material specifically identified.
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CBR (California Bearing Ratio) of Laboratory-Compacted Soils

Materials Tested in Accordance with ASTM Designation D 1883 - 99

Report Date:	April 27, 2	April 27, 2012 Snyder, Secary & Associates, LLC.				Gettysburg HACC					
Client:	-			er:		120008501					
Date Tested:		April 27, 2021									
Compaction Metho	d:	Standard				Load-Penetration Curve					
Sample ID:		120008501-S1									
Sample Descriptior	n: Red to brow	n lean clay with sand			70.0						
Sample Maximum	Dry Density:		110.6 lbs/ft3			· · · · · · · · · · · · · · · · · · ·					
Sample Optimum N	Density of Sample Before Soaking: sture Content of Sample Before Soaking: apaction of Test Sample Before Soaking: Density of Sample After Soaking:		15.3 %		60.0	00					
Specified Percenta	becified Percentage of Compaction:		95.0 %								
Dry Density of Sam	ple Before Soa	aking:	107.0 lbs/ft3								
Moisture Content o	of Sample Befo	re Soaking:	17.2 %	50.0							
Compaction of Test	t Sample Befor	re Soaking:	96.7%								
Dry Density of Sam	ple After Soak	ing:	97.2 lbs/ft ³	Stress on Piston in Ibs/in ² 0.06							
Moisture Content o	y Density of Sample After Soaking:		26.2 %	n lộ	40.0						
Compaction of Test	y Density of Sample After Soaking: bisture Content of Sample After Soaking: Impaction of Test Sample After Soaking:		87.9%	ton i							
Swell of Sample (%	of Initial Sam	ole Height):	2.94%	Pis							
Length of Time Sar	nple was Soak	ed:	96.0 Hrs	ss or	30.0						
Surcharge Amount:				Stree							
Special Sample Pre	Special Sample Preparation and/or Testing Procedures L		Used:		20.0	00					
Bearing Ratio of Sa	ample @ 0.100	of Penetration:	2.67		10.0						
Bearing Ratio of Sa	ample @ 0.200	of Penetration:	2.44		10.0						
Bearing Ratio of Sa	ample @ 0.300	of Penetration:	2.46								
Bearing Ratio of Sa	ample @ 0.400	of Penetration:	2.46		0.0	0.000 0.100 0.200 0.300 0.400 0.500					
Bearing Ratio of Sa	ample @ 0.500	of Penetration:	2.44			Penetration in Inches					
California Bearing I	Ratio of Sampl	e:	2.67								

The results stated on this report relate only to the material specifically identified.

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telecommunications | environmental | geotechnical 6520 Stonegate Drive, Suite 110, Allentown, Pennsylvania 18106 (610) 366-7120 (610) 366-7121 (fax) N:\project\2012\1200085 - Gettysburg HACC Pavement Design (S&S)\01\Lab testing\CBR-S1

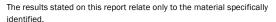


CBR (California Bearing Ratio) of Laboratory-Compacted Soils

Materials Tested in Accordance with ASTM Designation D 1883 - 99

Report Date:	April 27, 2012	Project:	Gettysburg HACC
Client:	Snyder, Secary & Associates, LLC.	Project Number:	120008501

Date Tested:	April 27, 2021						
Compaction Method:	Standard		L	_oad-Pe	netrati	ion Cur	ve
Sample ID:	120008501-S2						
Sample Description: Red Br	rown Lean Clay with Sand -	(CL)	140.00				
Sample Maximum Dry Dens	sity:	101.4 lbs/ft3					
Sample Optimum Moisture	Content:	21.1 %	120.00				
Specified Percentage of Compaction:		101.4 %					
		97.6 lbs/ft ³					/
Moisture Content of Sample Before Soaking:		17.9 %	100.00				
Compaction of Test Sample Before Soaking:		96.3%				/	·
Dry Density of Sample After	Soaking:	#VALUE!	Stress on Piston in lbs/in ² 00'09 00'08				
Moisture Content of Sample	e After Soaking:		<u>ă</u> 80.00 L				
Compaction of Test Sample	After Soaking:	#VALUE!	ton				
Swell of Sample (% of Initial Sample Height):		1.25%	Pis				
Compaction of Test Sample After Soaking: Swell of Sample (% of Initial Sample Height): Length of Time Sample was Soaked:		96.0 Hrs	ບິ 60.00 ຮູ		/		
Surcharge Amount:			Stres		, 		
Special Sample Preparatior	n and/or Testing Procedure	es Used:	40.00				
Bearing Ratio of Sample @	0.100" of Penetration:	4.33		[
Bearing Ratio of Sample @ 0.200" of Penetration:		4.67	20.00				
Bearing Ratio of Sample @	0.300" of Penetration:	4.61					
Bearing Ratio of Sample @	0.400" of Penetration:	4.64	0.00	0.100	0.200	0.300	0.4
Bearing Ratio of Sample @	0.500" of Penetration:	4.62				on in Inc	
California Bearing Ratio of S	Sample:	4.67	Penetra				1163



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